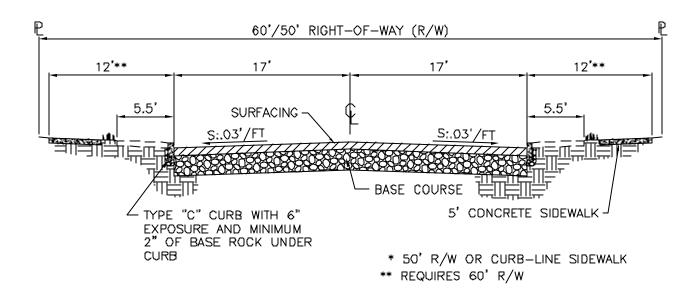
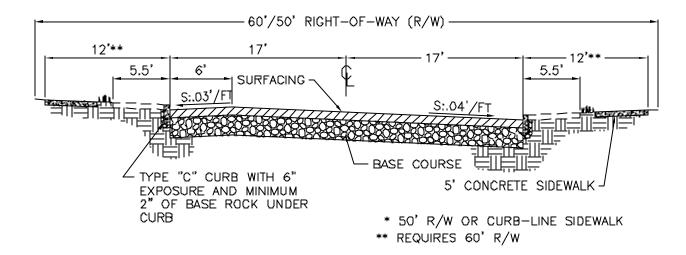
STANDARD CROSS SECTION

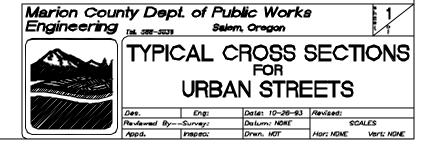


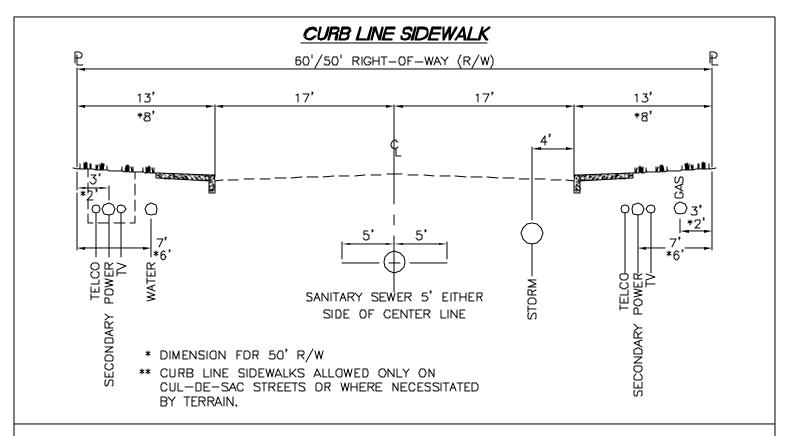
TILTED CROSS SECTION



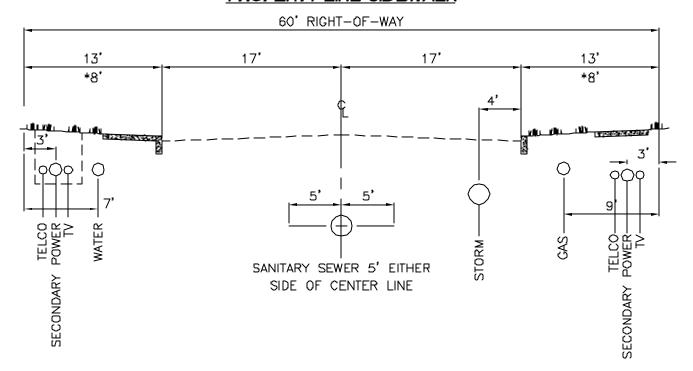
<u>NOTE:</u>

SURFACING AND BASE COURSE SHALL CONFORM TO MARION COUNTY D.P.W. ENGINEERING STANDARDS.





PROPERTY LINE SIDEWALK



NOTES:

- 1. TELCO, POWER, T.V., WATER SOUTH AND WEST SIDE GAS NORTH SIDE
- 2. TELCO, POWER, T.V., WATER MINIMUM COVER 30 INCHES

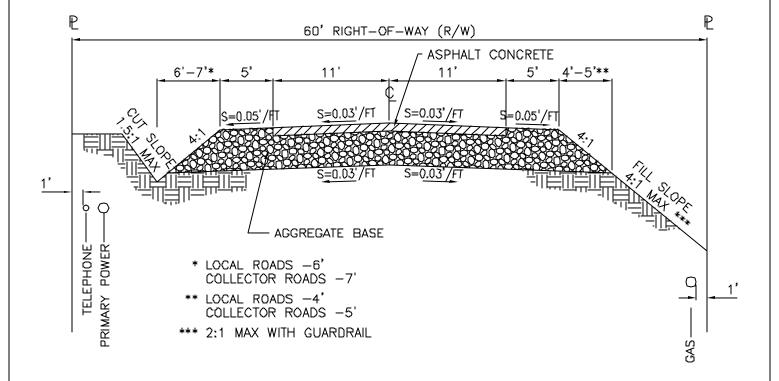




STANDARD UTILITY LOCATION
FOR
URBAN STREETS

2

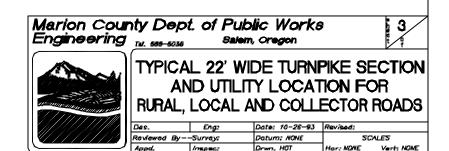
ı	Des.	Engr	Date: 10-26-93	Revised:	
J	Reviewed By	-Survey:	Datum: NONE	SCALES	
,	Appd.	Inspec:	Drwn. HOT	Har: NONE	Vert: NONE

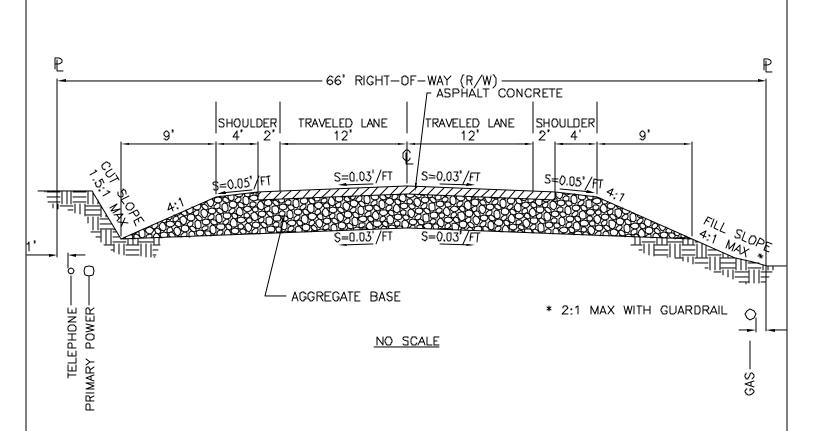


- 1. ASPHALT CONCRETE AND AGGREGATE BASE SHALL CONFORM TO MARION COUNTY D.P.W. ENGINEERING STANDARDS.
- 2. UTILITIES MAY BE PLACED IN SHOULDER AREA IF CUT OR FILL SLOPES EXTEND OUTSIDE OF R/W.

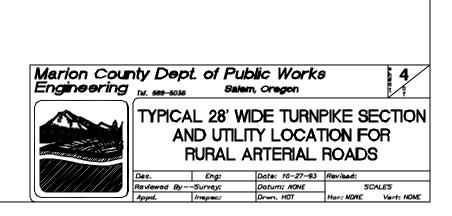
NO SCALE

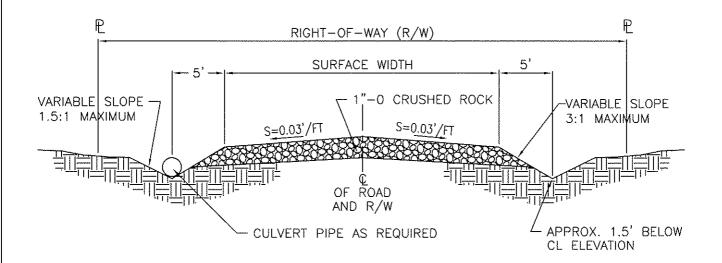
- 3. IN NEW SUBDIVISIONS: TELCO AND POWER -SOUTH AND WEST SIDE; GAS -NORTH AND EAST SIDE.
- 4. TELCO, POWER, GAS -MINIMUM COVER 30 INCHES.
- 5. 50' R/W WIDTH IS PERMITTED ON CUL-DE-SACS LESS THAN 500' LONG.





- 1. ASPHALT CONCRETE AND AGGREGATE BASE SHALL CONFORM TO SECTION IV OF THE MARION COUNTY D.P.W. ENGINEERING STANDARDS.
- 2. UTILITIES MAY BE PLACED IN SHOULDER AREA IF CUT OR FILL SLOPES EXTEND OUTSIDE OF R/W.
- 3. IN NEW SUBDIVISIONS: TELCO AND POWER -SOUTH AND WEST SIDE; GAS -NORTH AND EAST SIDE.
- 4. TELCO, POWER, GAS -MINIMUM COVER 30 INCHES.

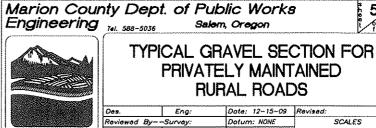




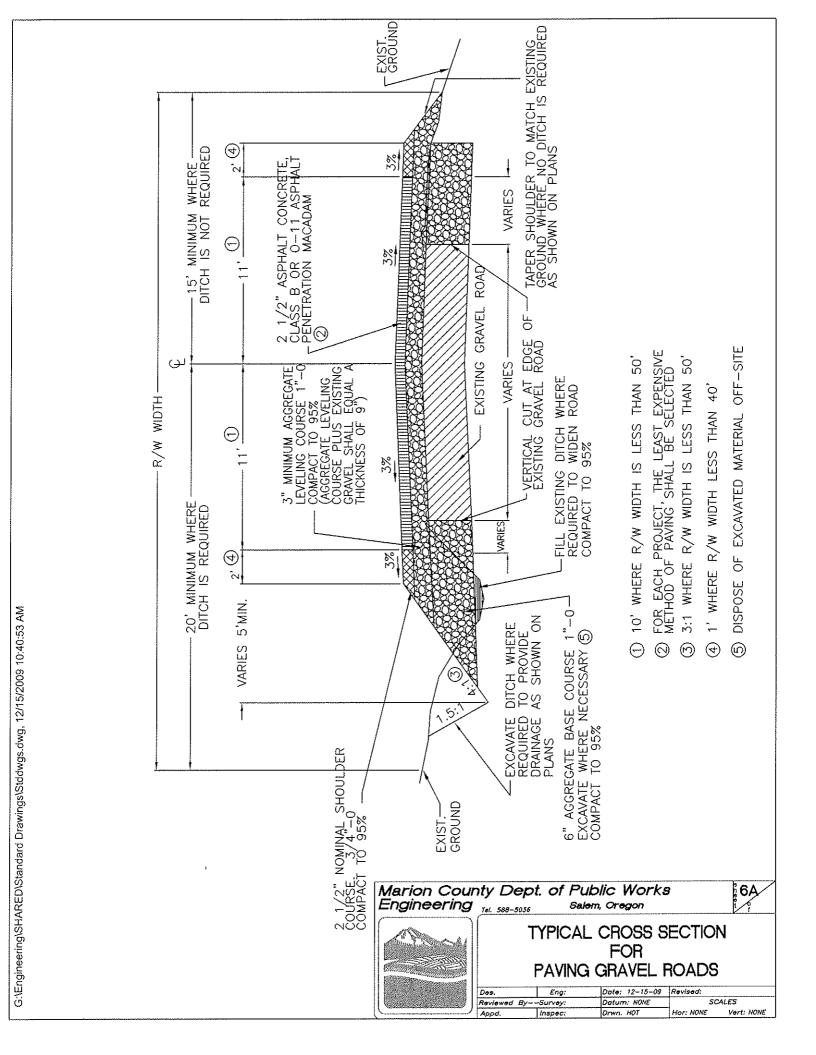
1. SURFACE WIDTH AND CRUSHED ROCK SHALL CONFORM TO THE FOLLOWING:

NUMBER OF <u>DWELLINGS SERVED</u>	SURFACE WIDTH (FEET)	1"—0 CRUSHED ROCK THICKNESS (INCHES)
1	10'	6"
2 OR MORE	16'	8"

2. UTILITIES SHALL BE LOCATED AS SHOWN ON TYPICAL 22' TURNPIKE SECTION FOR RURAL ROADS (MARION COUNTY STANDARD DRAWING NUMBER 3)

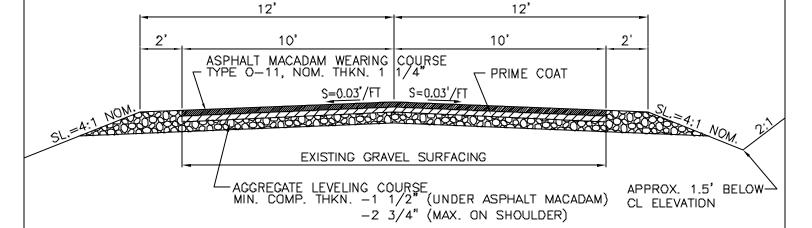


SCALES Vert: NONE



ASPHALT PENETRATION MACADAM (O-11)

TYPICAL SECTION



WHERE THE MACADAM PAVEMENT IS WIDER THAN THE EXISTING GRAVEL SURFACE, THE AGGREGATE LEVELING COURSE SHALL HAVE A MINIMUM THICKNESS OF 6"

MINIMUM

	ASPHALT		AGG	REGATE	
	TYPE GALS./S.Y.		GRAVEL	CU.YDS./S.Y.*	
PRIME COAT	MC-250	0.55	3/4"-0	0.019	
1ST SPREAD	CRS-2	0.45	1 1/4"-3/4"	0.019	
2ND SPREAD	CRS-2	0.55	3/4"-1/2"	۵.019	
3RD SPREAD	CRS-2	0.55	1/2"-1/4"	0.014	
SEAL COAT	CRS-2	0.45	1/4"-#10	0.009	

^{*} MEASURED IN TRUCK

THE CATIONIC EMULSION ASPHALT SHALL MEET THE SPECIFICATIONS OF AASHTO M208-72 OR ASTM D2397-73. AGGREGATES SHALL BE IN DESIGNATED SIZES AND SHALL CONFORM TO THE REQUIREMENTS OF SUBSECTION 703.12 OF THE STANDARD SPECIFICATIONS OF THE OREGON STATE HIGHWAY DEPARTMENT, CURRENT ISSUE.

NOTES:

- 1. THE MINIMUM THICKNESS OF THE AGGREGATE LEVELING COURSE MAY HAVE TO BE INCREASED, DEPENDING ON THE THICKNESS AND CONDITION OF THE EXISTING GRAVEL SURFACE.
- 2. CONSTRUCTION OF DRAINAGE CULVERTS, ROADSIDE DITCHES AND/OR ELEVATION OF ROADWAY MAY BE REQUIRED.

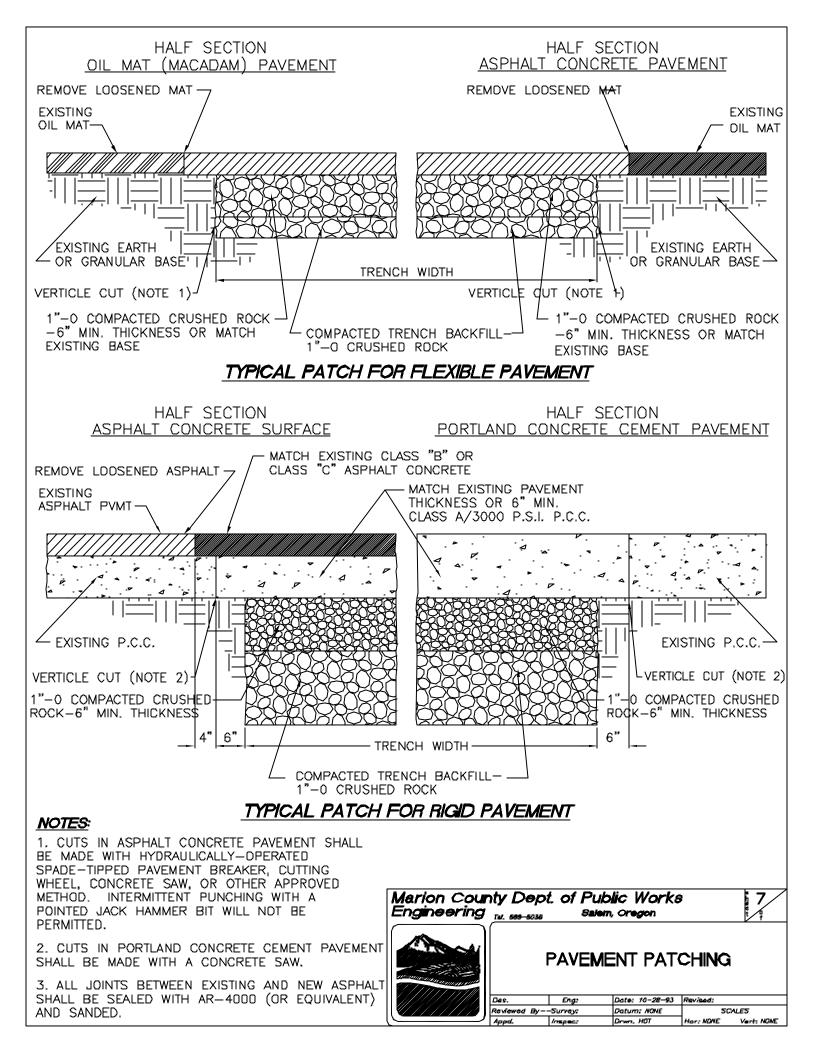


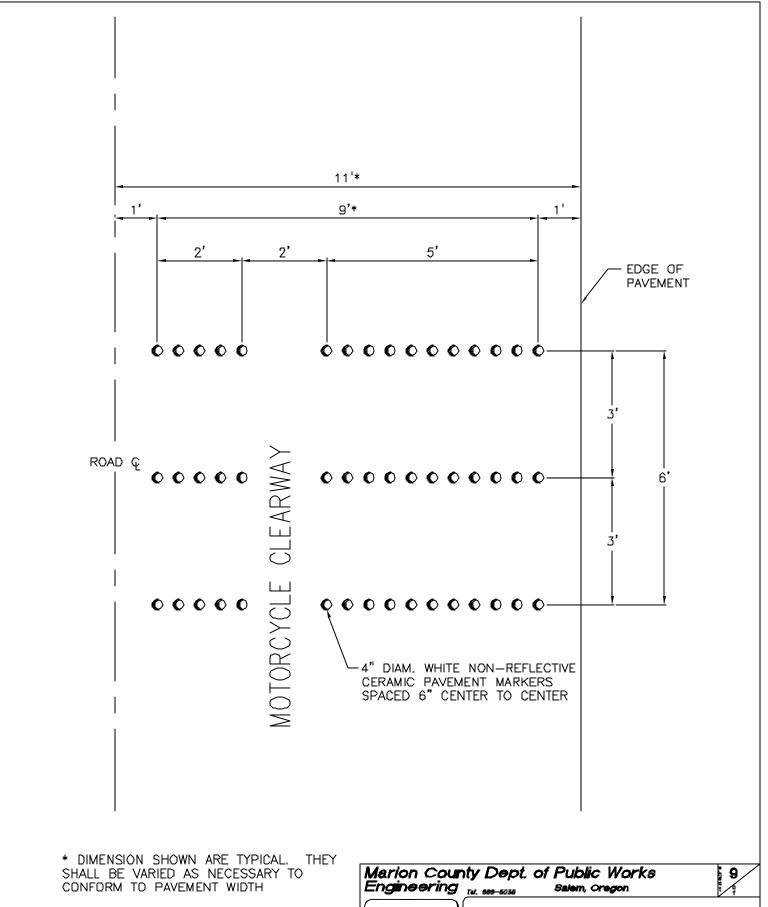


TYPICAL ASPHALT
PENETRATION MACADAM (O-11)
SURFACING DETAILS

6B

	Das.	Eng:	Date: 10-27-93	Revised	
ı	Reviewed By Survey:		Datum; NONE	5CALE5	
_	Appd.	Inspec:	Drwn. HOT	Har: NONE Vert: NO	WE

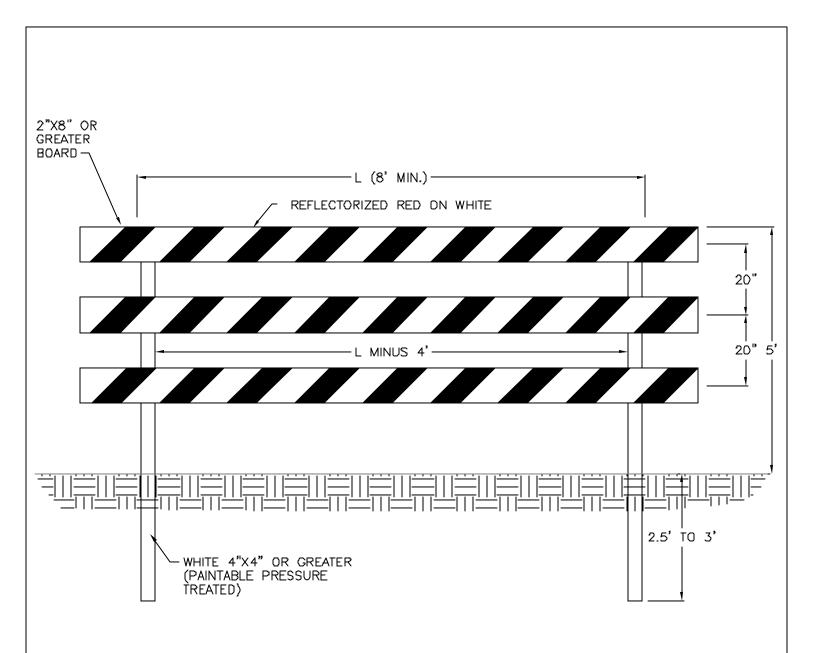




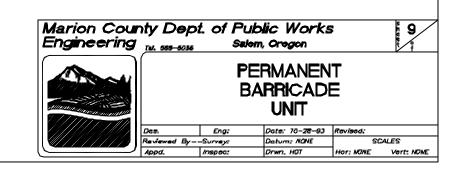


RUMBLE	STR	IPS
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Das.	Eng:	Date: 10-28-93	Revised:	
Reviewed By Survey:		Datum: NONE	SCALES	
Appd.	ppd. Inspec: Drwn. HOT		Har: NONE	Vert: NONE

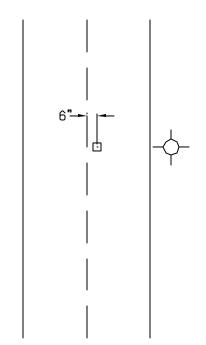


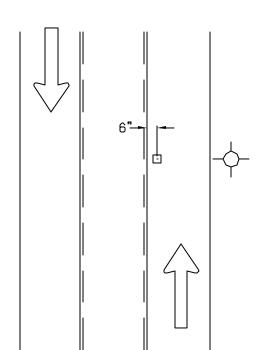
- 1) REFLECTORIZED SHEETING SHALL BE ENGINEERING GRADE, CONFORMING TO THE OSHD "STD. SPECS, FOR HWY. CONSTRUCTION" 02910
- 2. THE LENGTH OF THE BARRICADE UNIT "L" NORMALLY WILL BE SHOWN ON PROJECT PLANS OR PERMITS. WHEN NO "L" IS SHOWN, THE 8' MIN. SHALL BE USED

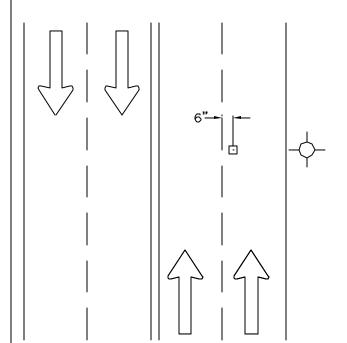


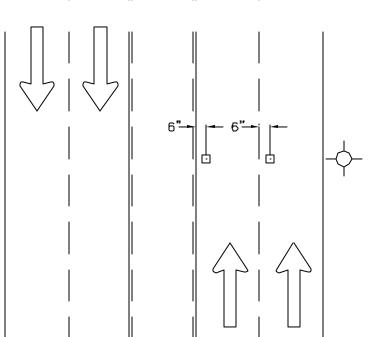
2 LANE ONE-WAY OR TWO-WAY

2 LANE TWO-WAY CONTINUOUS LEFT TURN LANE



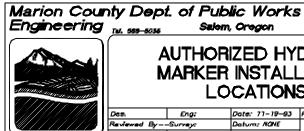






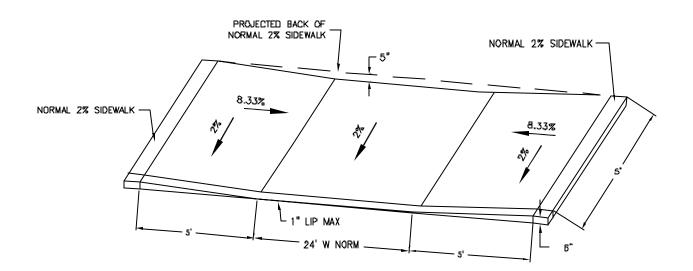
4 LANE TWO-WAY **UNDIVIDED**

4 LANE TWO-WAY CONTINUOUS LEFT TURN LANE

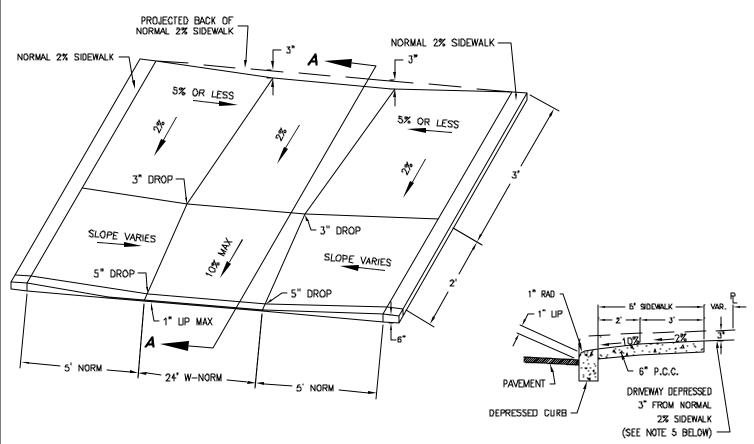


AUTHORIZED HYDRANT
MARKER INSTALLATION
LOCATIONS

ı	Des.	Eng:	Date: 71-19-93	Revised:	
ı	Reviewed By Survey:		Datum: NONE	SCALES	
_	Appd.	Inspec:	Drwn. HOT	Hor! NONE	Vert: NONE



DRIVEWAY APPROACH WITH CURB-LINE WALK OPTION #1 OF 3 - 2% CROSS SLOPE



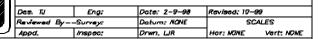
DRIVEWAY APPROACH
WITH CURB-LINE WALK
OPTION \$2 OF 3 - SPLIT 2X/10% CROSS SLOPE

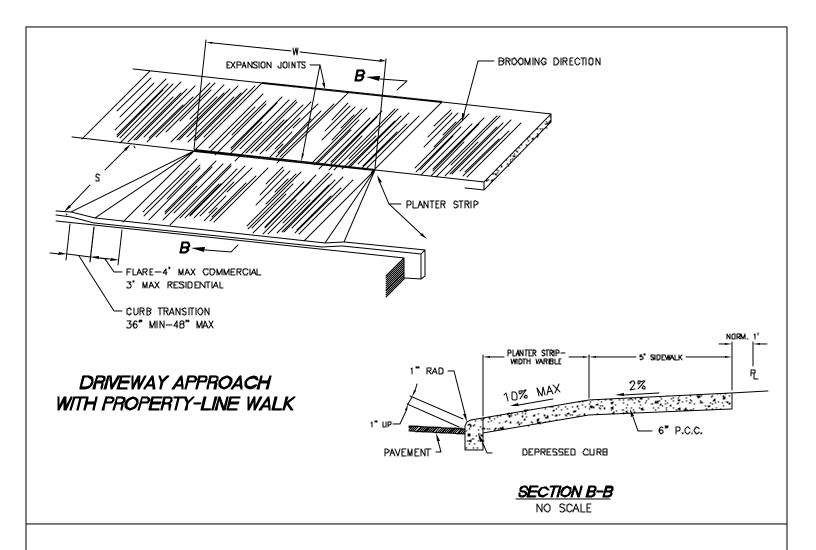


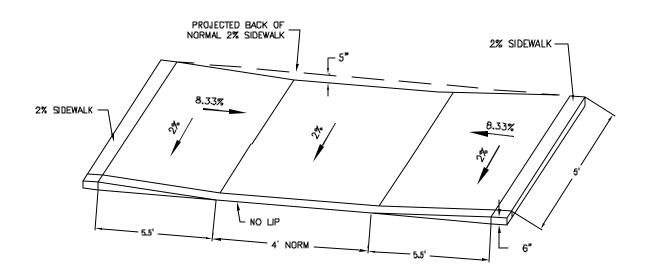
£ 11 🙊



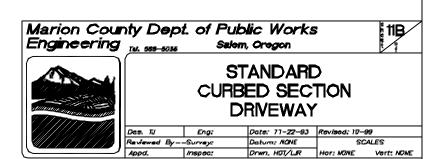
STANDARD DRAWING ACCESSIBLE ROUTE DRIVEWAYS DETAIL

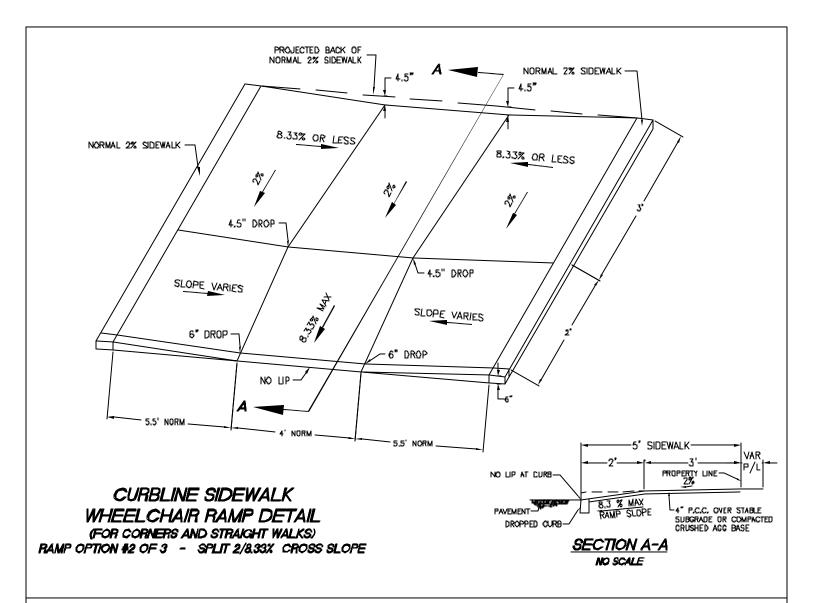


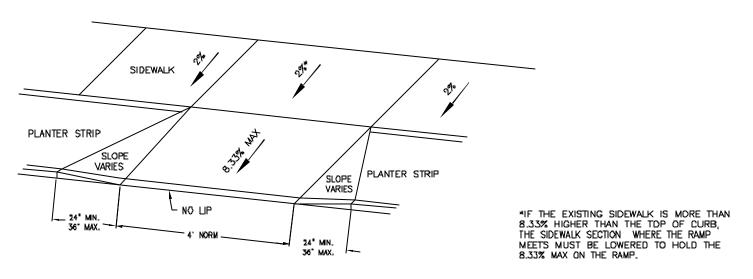




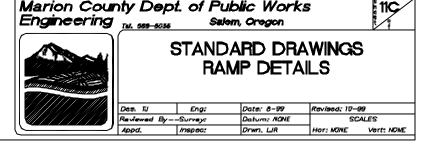
CURBLINE SIDEWALK
WHEELCHAIR RAMP DETAIL
(FOR CORNERS AND STRAIGHT WALKS)
RAMP OPTION #1 OF 3 - 2% CROSS SLOPE



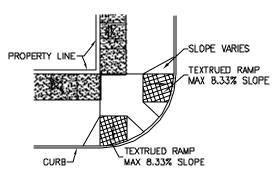




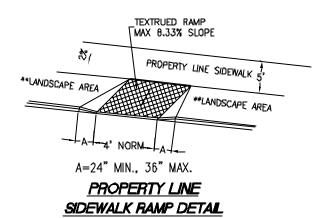
PROPERTY-LINE SIDEWALK WHEELCHAIR RAMP DETAIL RAMP OPTION #3 OF 3 - 8.33% MAX CROSS SLOPE



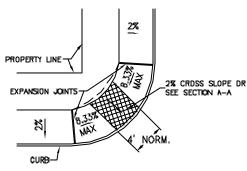
Marion County Dept. of Public Works



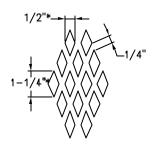
END RAMPS (PROPERTY-LINE WALKS)



** IF THE "LANDSCAPED AREA" IS PAVED, TREAT CURB TRANSITION LENGTH AND SURFACE TEXTURING LIMITS THE SAME AS CURBLINE SIDEWALK RAMP DETAIL.



CENTER RAMP (CURB-LINE WALKS)



* NOMINAL DIMENSIONS

RAMP TEXTURE DETAIL

LEGEND

----- EXPANSION JOINTS

PORTION OF RAMP THAT IS TEXTURED

NOTE:

RAMP TEXTURING IS TO BE DONE WITH AN EXPANDED METAL GRATE PLACED AND REMOVED FROM WET CONCRETE TO LEAVE A DIAMOND PATTERN AS SHOWN OR APPROVED EQUAL DIAMOND PATTERN. THE LONG AXIS OF THE DIAMOND PATTERN SHALL BE PERPENDICULAR TO THE CURB. GROOVES SHALL BE 1/8" DEEP AND 1/4" WIDE. THE TEXTURED PORTION OF THE RAMP SHALL BE 5 FEET MINIMUM LENGTH MEASURED PERPENDICULAR TO THE CURB.

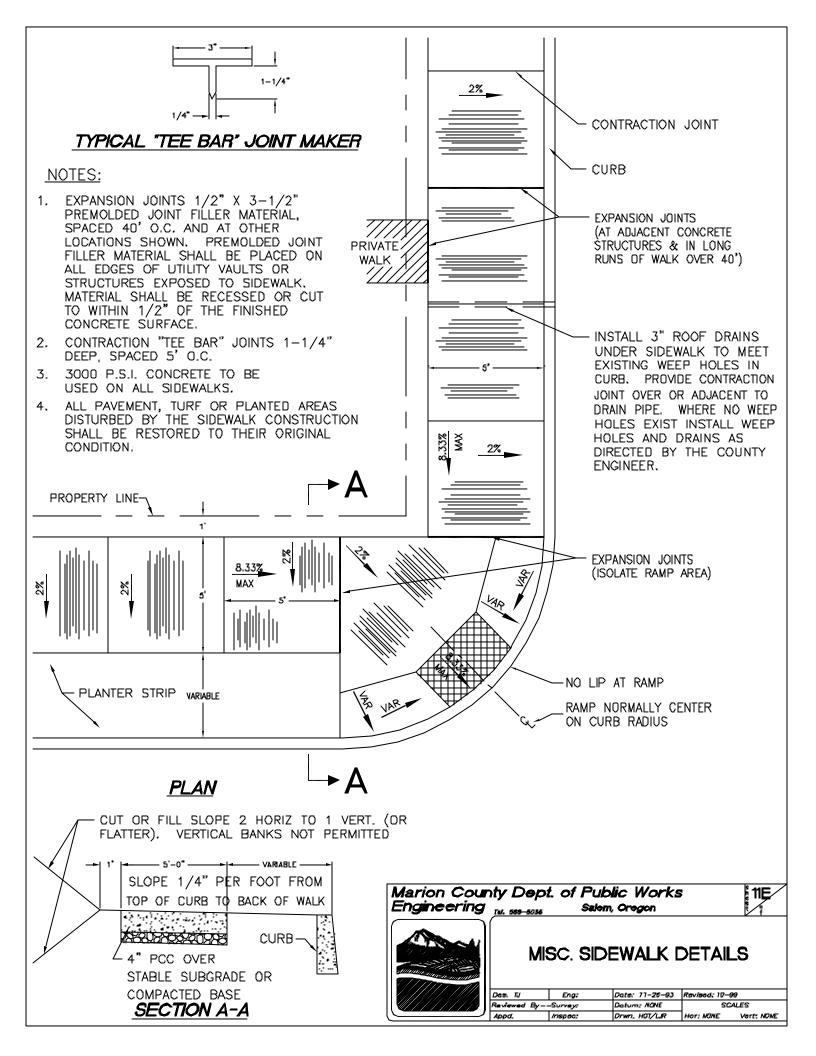




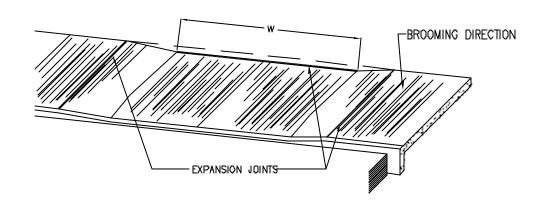
WHEELCHAIR RAMP LAYOUT AND TEXTURING

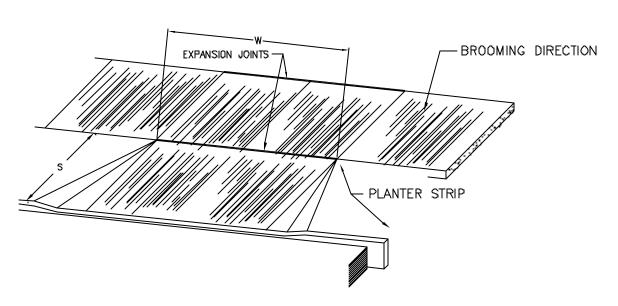
11D

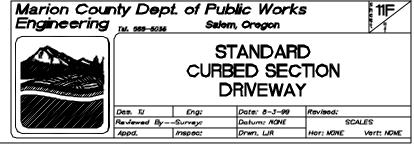
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Reviewed By Survey:		Datum: NONE	SCALES	
Appd. Inspec:		Drwn. LIR	Hor: NONE	Vert: NOME



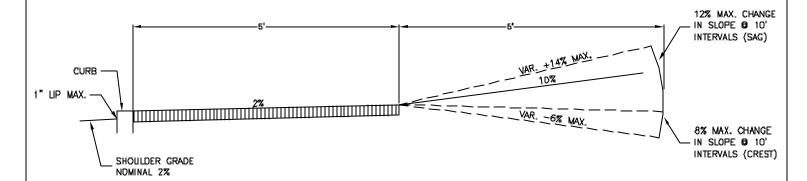
- A. APPROACH WIDTH (W): RESIDENTIAL 12'-24' COMMERCIAL* 28'-40'
 - * FOR COMMERCIAL DRIVEWAYS, W SHALL BE SET BY THE DIRECTOR OF PUBLIC WORKS ON A SITE SPECIFIC BASIS.
- B. FLARE
 - A. 36" FOR COMMERCIAL AND INDUSTRIAL WHERE TRAVELED LANE IN STREET IS ADJACENT TO THE CURB. (I.E. PARKING PROHIBITED).
 - B. NONE REQUIRED FOR RESIDENTIAL AND COMMERCIAL WHERE PARKING IS ALLOWED IN STREET ADJACENT TO CURB.
- C. BROOMING DIRECTIONS



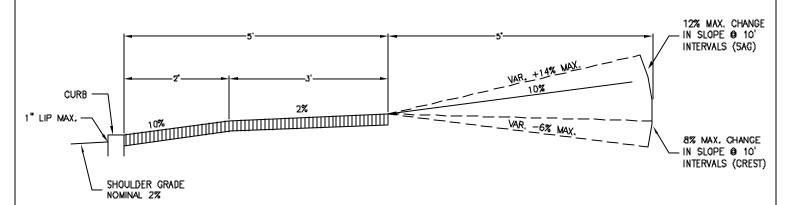




D. MATCHING EXISTING DRIVEWAY SLOPES.



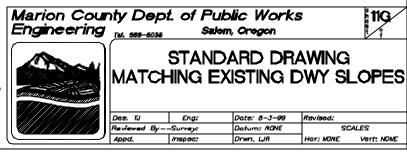
2% SIDEWALK -5% STREET SLOPE +7% MATCHING GRADE=12% MAX.



SPLIT SLOPE SIDEWALK

NOTES:

WHEN EXISTING DRIVEWAY CANNOT BE MATCHED TO THE NEW APPROACH WITHIN SLOPE LIMITATION AS SHOWN, ADJUST THE EXISTING DRIVEWAY; NOT THE CURB, APPROACH OR SIDEWALK.



E. CURB REMOVAL

WHEN AN EXISTING FULL HEIGHT CURB SECTION IS REMOVED FOR CONSTRUCTION, THE FOLLOWING PROVISIONS SHALL APPLY:

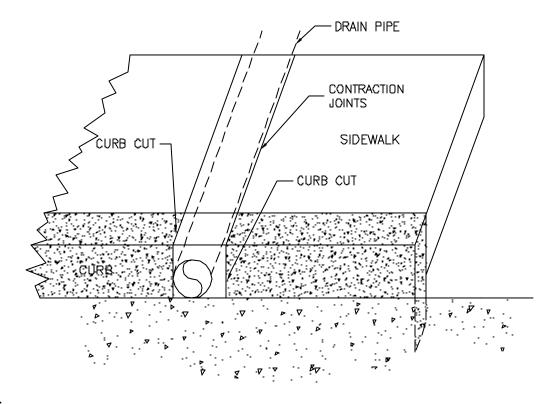
- 1. VERTICAL SAW CUTS SHALL BE MADE OUTSIDE THE EDGES OF THE CURB TRANSITIONS (THIS APPLIES TO BOTH TYPE A AND C CURBS). WHERE WEEP HOLES ARE TO BE INSTALLED, ADDITIONAL CURB WILL NEED TO BE REMOVED.
- 2. TYPE A "CURB AND GUTTERS": THE ENTIRE CURB AND GUTTER SHALL BE REMOVED AND REPOURED; OR, A LONGITUDINAL VERTICAL CUT SHALL BE MADE AT THE FLOW LINE OF THE GUTTER LEAVING THE APRON IN TACT. ALL MATERIAL IN THE CURB AREA SHALL BE REMOVED TO SUBGRADE AND REPOURED. PROVIDE 6" OF CONCRETE BELOW THE FLOW LINE IN THE CURB AREA.
- 3. TYPE C CURB: THE ENTIRE CURB SHALL BE REMOVED TO FULL DEPTH AND REPOURED; OR , THE CURB SHALL BE BROKEN OUT FROM A POINT 2" BELOW THE ADJACENT ASPHALT DOWNWARD AND AWAY FROM THE STREET AT NO LESS THAN A 45" ANGLE AND REPOURED. PROVIDE 10" OF CONCRETE BELOW THE ASPHALT IN THE CURB AREA.

F. DRAIN LINES

WEEP HOLES FOR DRAINS ARE TO EXIT IN A FULL HEIGHT CURB SECTION OUTSIDE THE CURB TRANSITION AREA OF DRIVEWAY. THE DRAIN LINES IN THE SIDEWALK ARE TO BE LOCATED UNDER OR ADJACENT TO A CONTRACTION JOINT. DRAIN LINES ARE TO CROSS SIDEWALKS AT 90° (PERPENDICULAR) TO THE SIDEWALK, NOT ASKEW.

WHERE CURB CUTS ARE MADE FOR CONSTRUCTION OF A DRIVEWAY APPROACH, ONE DRAIN LINE IS ALLOWABLE IN THE CURB TRANSITION AREA IF THE LINE IS PLACE DIRECTLY ADJACENT TO THE CURB CUT (HIGHEST POINT IN THE TRANSITION).

ALL DRAIN LINES ARE TO BE PLACED DOWN AT THE FLOW LINE OF THE GUTTER.



NOTE:

DRAIN PIPE IS TO BE PLACED ADJACENT TO ONE OF THE CURB CUTS. A CONTRACTION JOINT IS TO BE SCORED ALONG BOTH CUTS. THE PIPE IS TO BE DOWN TO THE FLOW LINE OF THE GUTTER.



G. DEEP SCORING, SHINING & EXPANSION JOINTS

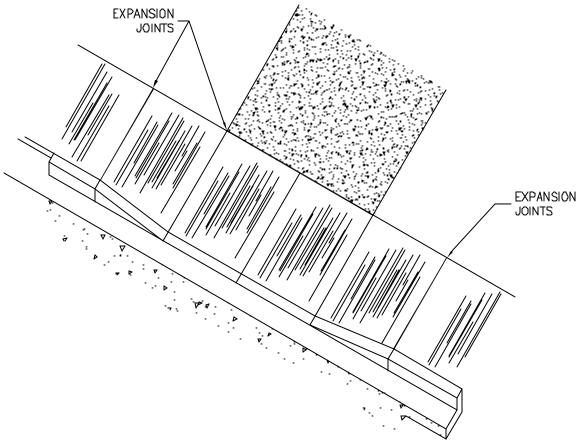
DEEP SCORED CONTRACTION JOINTS ARE TO BE MADE EVERY 5 FEET OF SIDEWALK AND EVERY 15 FEET OF CURB. WHERE THE CURB IS CUT TO ALLOW FOR A DRAIN LINE, THE DEEP SCORE IS TO BE MADE AT BOTH CURB EDGES, NOT OVER THE PIPE. SEE NOTE E ABOVE.

DEEP SCORED CONTRACTION JOINTS ARE TO BE FORMED TO A DEPTH OF 1/3 OF THE THICKNESS OF CONCRETE AND A WIDTH OF ABOUT 1/8 INCH.

AFTER BROOMING, TOOL ROUND & SHINE THE OUTER EDGE OF THE SIDEWALK (NOT THE CONTRACTION JOINTS) & THE OUTER EDGE OF THE DRIVEWAY IN THE PLANTER STRIP.

EXPANSION JOINTS USING 1/2" X 3 1/2" PRE-MOLDED JOINT FILLER MATERIAL ARE REQUIRED:

- A. ALONG THE SIDEWALK AT THE DRIVEWAY (A PROPERTY LINE SIDEWALK WILL REQUIRE EXPANSION JOINTS ON BOTH SIDES OF THE SIDEWALK);
- B, ALONG THE SIDEWALK WHERE IT INTERSECTS ANOTHER SIDEWALK;
- C. ON THE EDGES OF UTILITY VAULTS OR OTHER STRUCTURES EXPOSED TO THE SIDEWALK;
- D. IN THE SIDEWALK TO ISOLATE A WHEELCHAIR RAMP (SEE SHEET 5);
- E. ON CURB-SIDE SIDEWALKS PLACED AT 90 DEGREES ACROSS THE SIDEWALK AT THE BEGINNING (TOP) OF THE CURB TRANSITION: AND
- F. IN SIDEWALKS WITH THE LONG RUNS. NO RUNNING PIECE OF SIDEWALK SHALL BE MORE THAN 40' WITHOUT AN EXPANSION JOINT.



H. CONCRETE SPECIFICATIONS

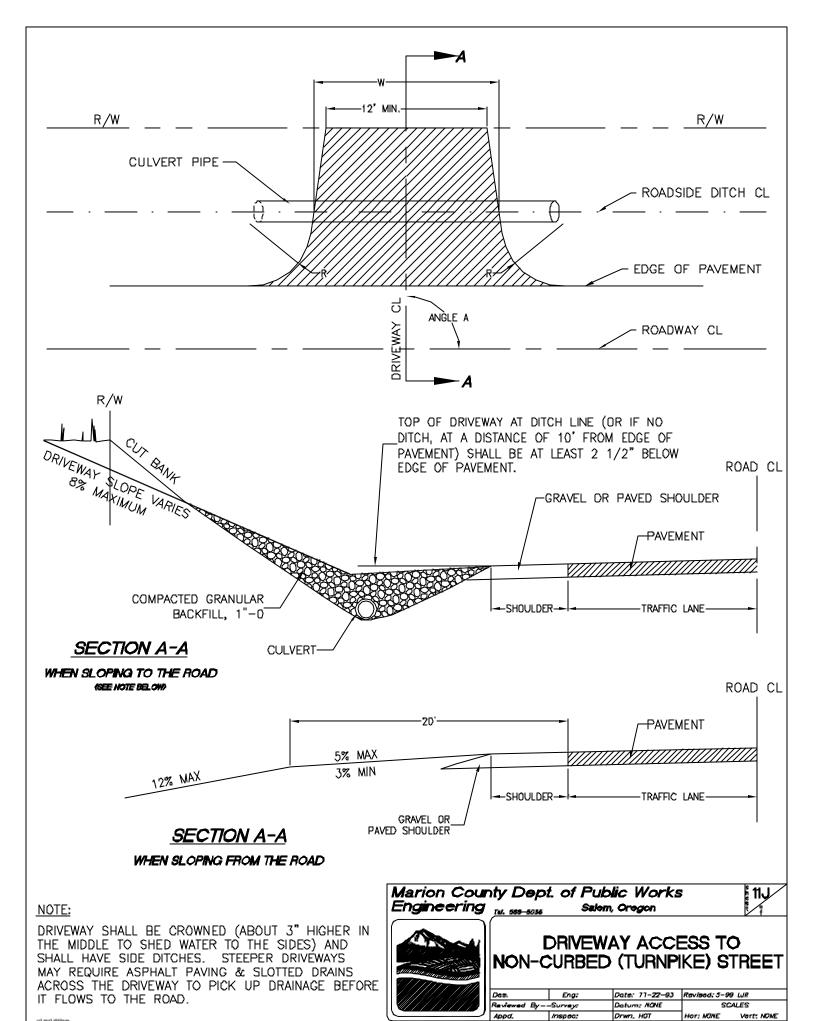
A MINIMUM OF 3,000 PSI CONCRETE SHALL BE USED FOR ALL CURBS, DRIVEWAY APPROACHES AND SIDEWALKS. CONCRETE SHALL BE AIR ENTRAINED, TOTAL AIR CONTENT (PERCENT BE VOLUME OF CONCRETE) SHALL BE NOT LESS THAN 5 PERCENT OR MORE THAN 7 PERCENT.

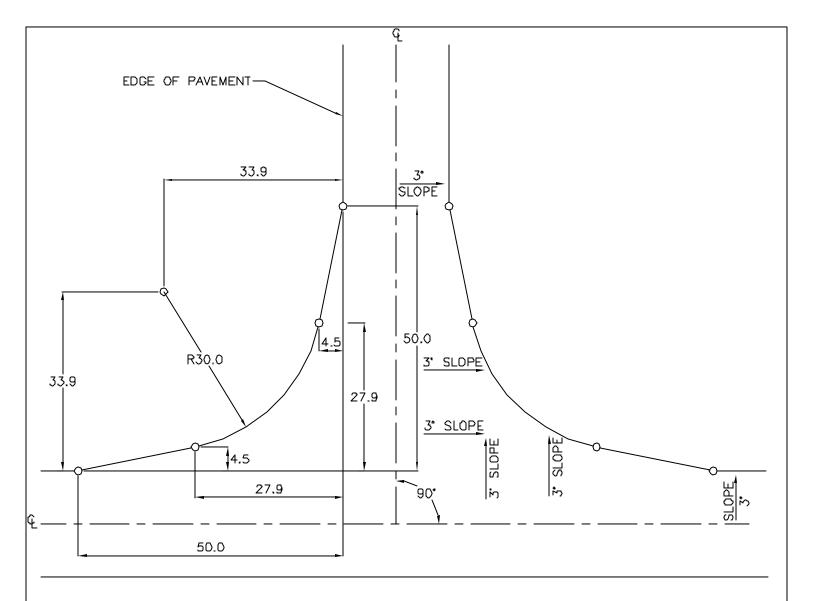
Marion County Dept. of Public Works Engineering [14, 588-6056] Salem, Oregon



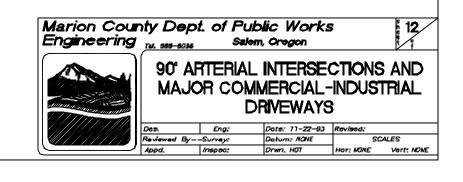
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	Reviewed By Survey:		Datum: NONE	SCAL	LES
	Appd.	Inspec:	Drwn. LIR	Hor: NONE	Vert: NOME

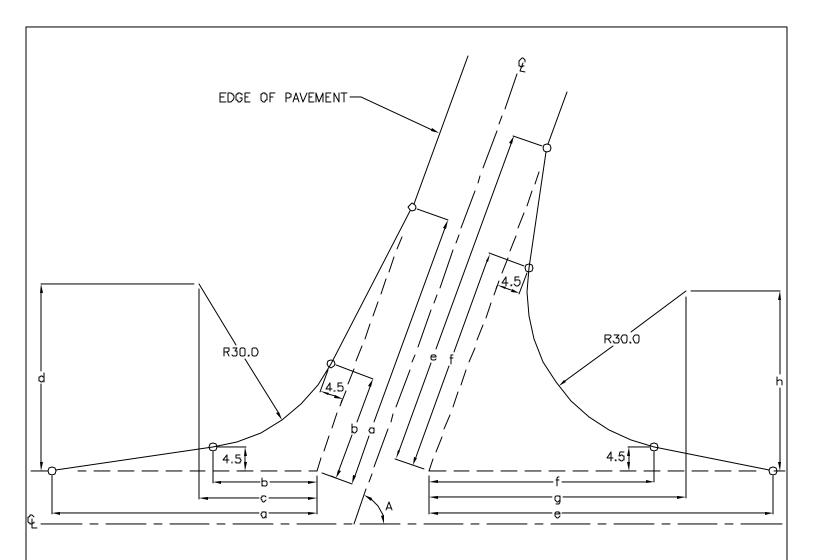
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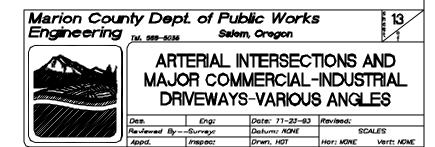
- 1. DIMENSIONS GIVEN ARE FOR A "9D ANGLE OF INTERSECTION. FOR ANGLES DEVIATING BY MORE THAN 5", SEE STANDARD DRAWING NUMBER 13.
- 2. SURFACING AND BASE COURSE OF THE INTERSECTION, INCLUDING THE FLARE AREA, SHALL CONFORM TO SECTION IV OF THE ENGINEERING STANDARDS.

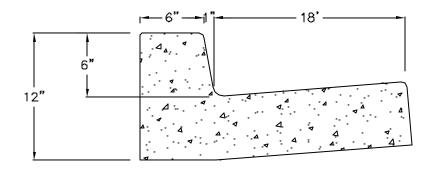




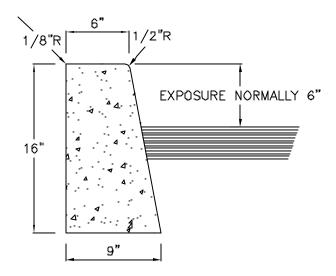
1. SURFACING AND BASE COURSE OF THE INTERSECTION, INCLUDING THE FLARE AREA, SHALL CONFORM TO SECTION IV OF THE ENGINEERING STANDARDS.

ANGLE A	а	۵	U	ਰ	ω	f	g	h
60°	50.0	15.9	19.8	34.2	74.0	52.5	58.7	33.9
70°	50.0	19.5	23.9	34.2	65.D	42.8	48.5	34.0
°08	50,0	23.6	28.6	34.1	57.0	34.5	40.4	33.9
90°	50.0	27.9	33.9	33.9	=0	=b	=¢	=d
100°	57.0	34.5	40.4	33.9	50.0	23.Б	28.6	34.1
110*	65.0	42.8	48.5	34.0	50.0	19.6	23.9	34.2
120°	74.0	58.7	58.7	33.9	50.0	15.9	19.8	34.2





TYPE "A" CURB AND GUTTER

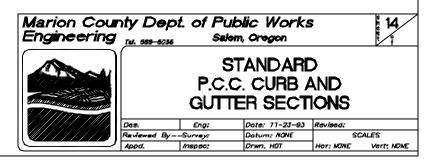


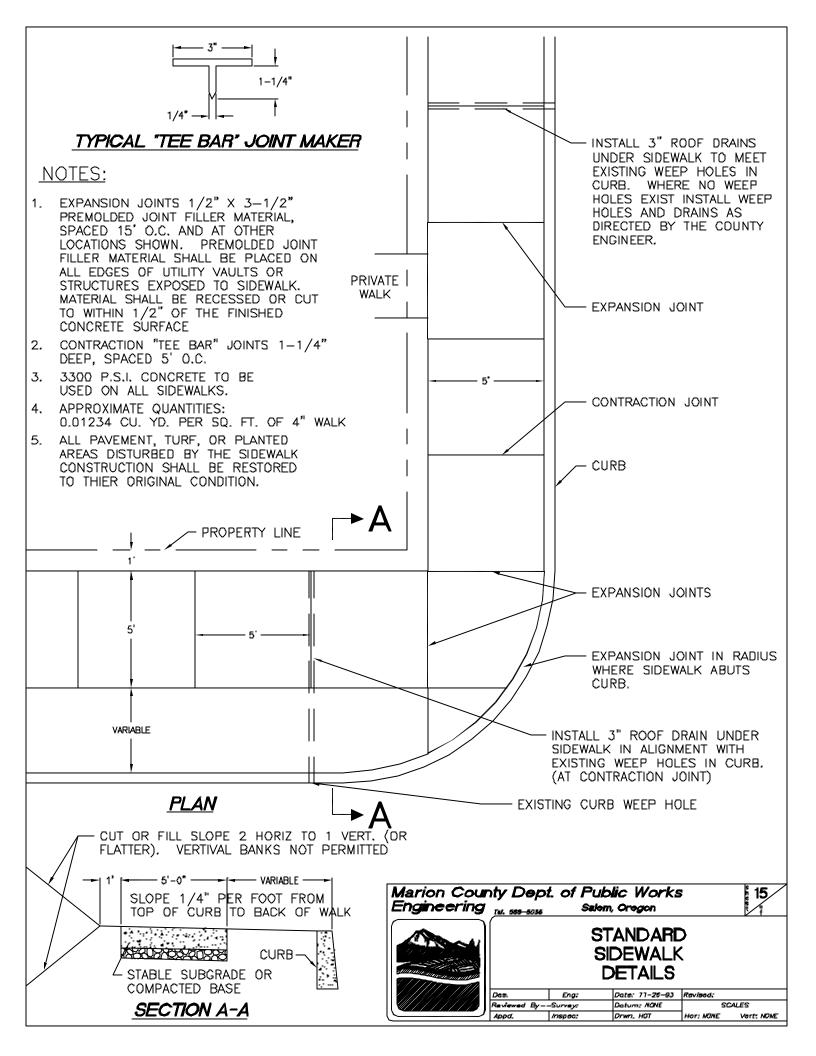
TYPE "C" CURB

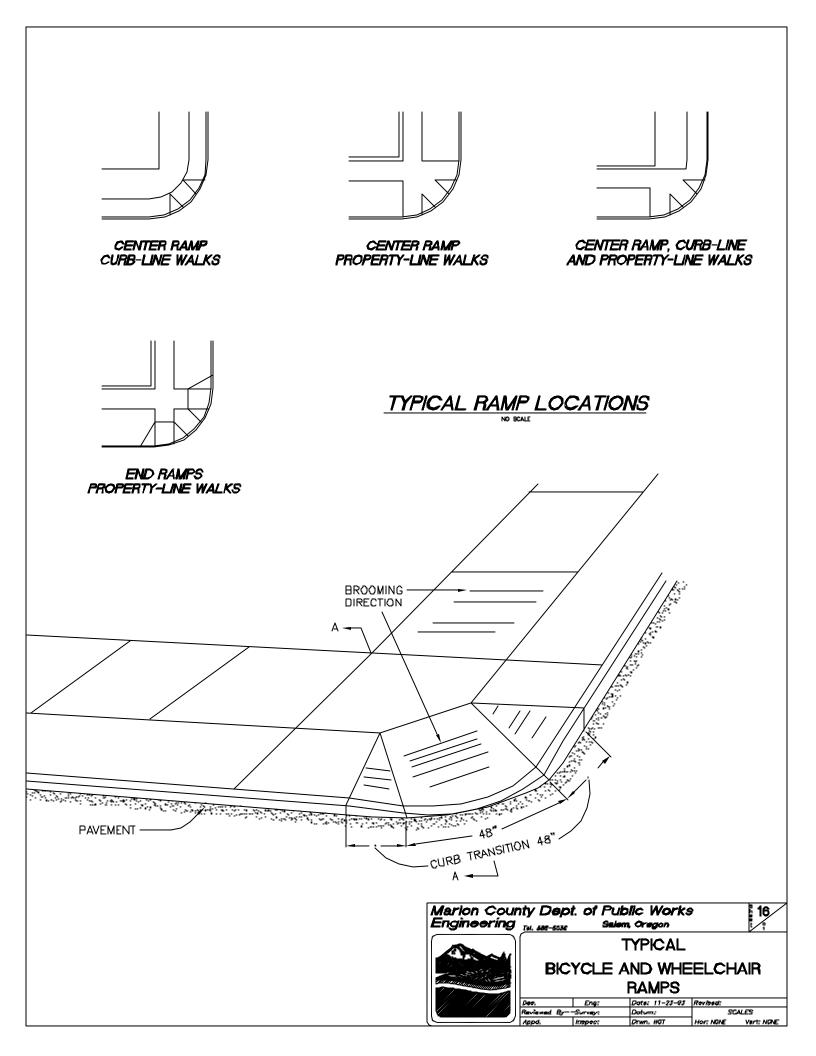
NOTES:

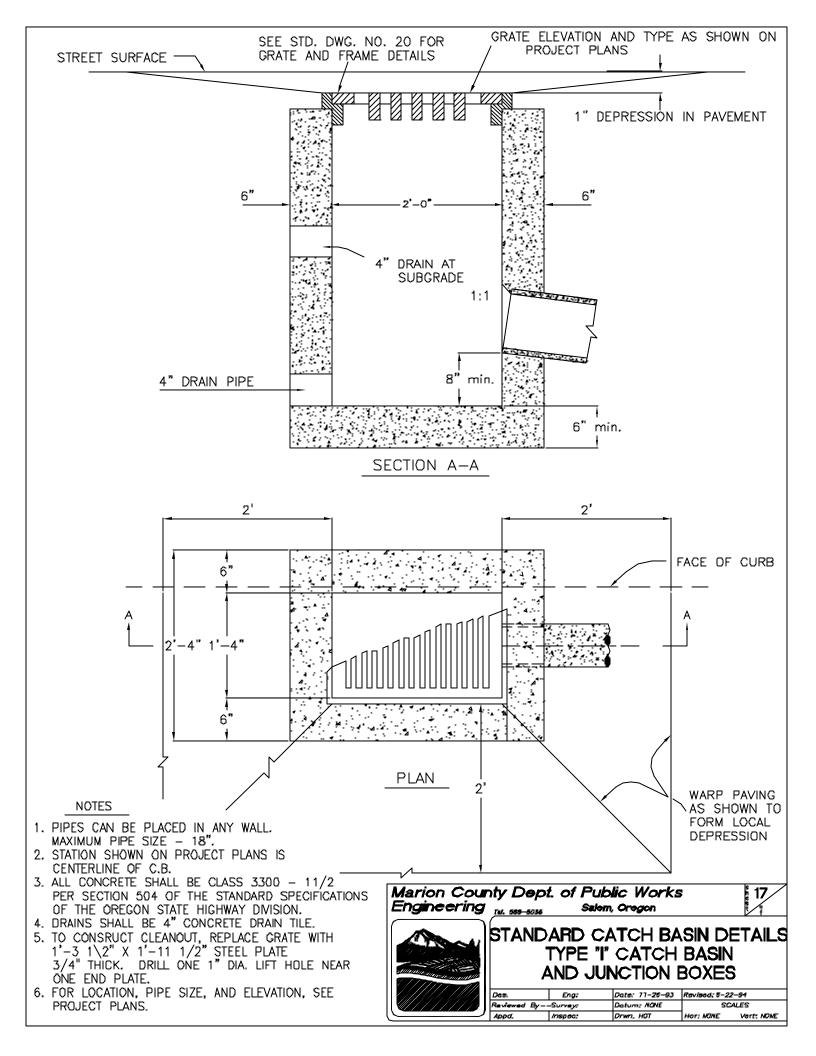
- 1. CURBS AND GUTTERS SHOWN MAY BE USED WITH EITHER A.C. OR P.C.C. PAVEMENTS.
- 2. TRANSITIONS FROM ONE TYPE CURB TO ANOTHER WILL BE DETAILED ON PROJECT PLANS AS NECESSARY.
- 3. "TEE-BAR" CONTRACTION JOINT TO BE INSTALLED EVERY 15' OF CURB.
- 4. 3300 P.S.I. CONCRETE TO BE USED FOR ALL CURBS.

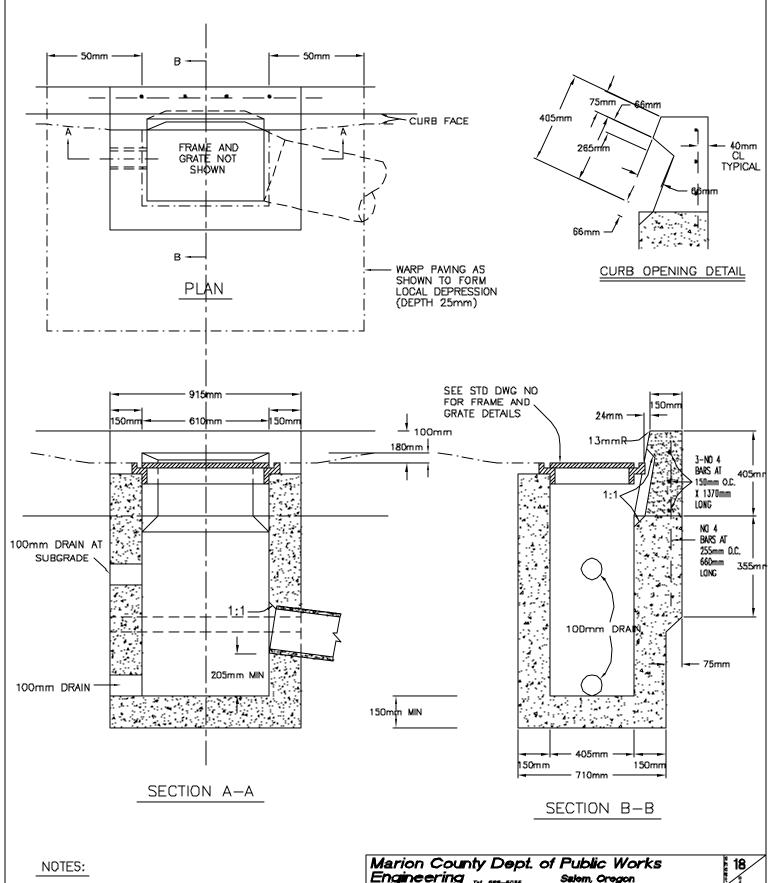
CURI	9 QUANTITIES
CU. YL). PER LIN. FT.
TYPE	TYPE
Α	0.04942
С	0.03085











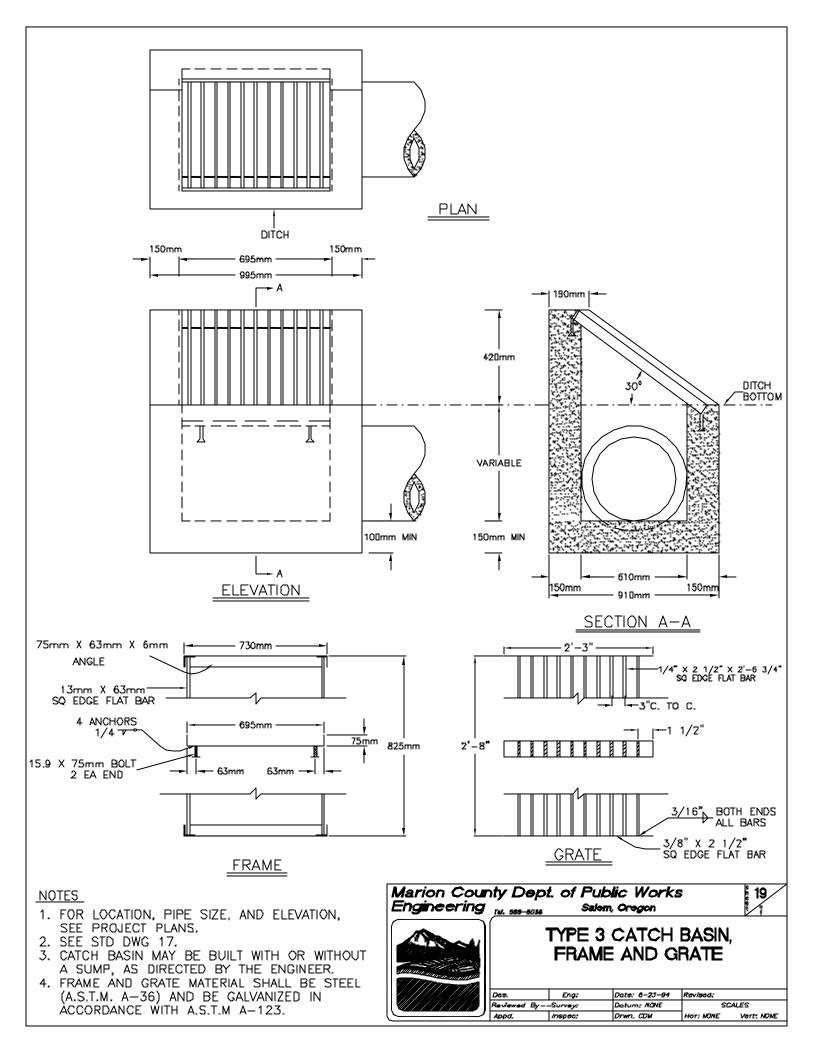
- 1. SEE STD. DWG. 17.
- 2. DRAINS SHALL BE 100mm CONCRETE DRAIN TILE.
- 3. FOR LOCATION, PIPE SIZE, AND ELEVATION, SEE PROJECT PLANS.
- 4. PIPES CAN BE PLACED IN ANY WALL. MAXIMUM PIPE SIZE 455mm.
- 5. STATION SHOWN ON PROJECT PLAN IS TO CENTERLINE OF CATCH BASIN.

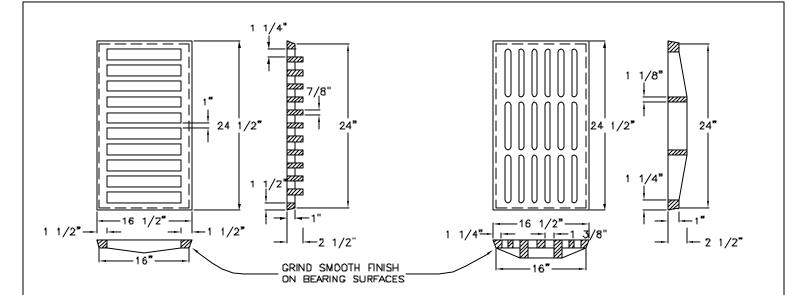
Engineering Salem, Oregon



STANDARD CATCH BASIN DETAILS TYPE 2 (SIDE INLET) **CATCH BASIN**

	Des.	Eng:	Date: 6-22-84	Revised:	
ı	Reviewed By	-Survey:	Datum: NONE	SCA	LES
_	Appd.	Inspec:	Drwn. CDM	Hor! NONE	Vert: NONE

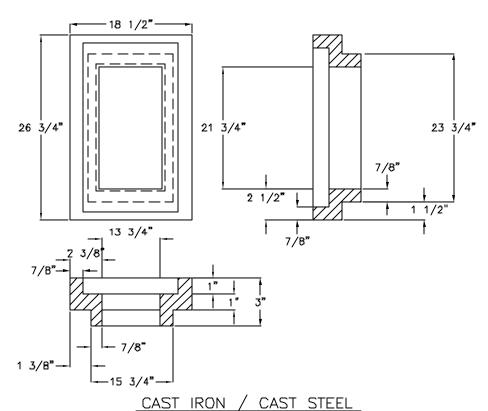




TYPE A GRATE 1-1/4" CLEAR SPACING

TYPE A GRATE 1-3/8" CLEAR SPACING

CAST IRON / CAST STEEL GRATES



GRATE FRAMES

NOTES

- 1. ALL CASTINGS SHALL CONFORM TO ASTM A-48
 (AASHTO M105) FOR GRAY IRON CASTINGS, CLASS 30,
 OR (AASHTO M192), CLASS 70, FOR CAST STEEL.
 2. ROUNDS, FILLETS, TAPERS AND OTHER MINOR
 MODIFICATIONS TO THE DIMENSIONS SHOWN FOR
 CASTINGS MAY BE MADE TO CONFORM TO COMMON SHOP PRACTICES.

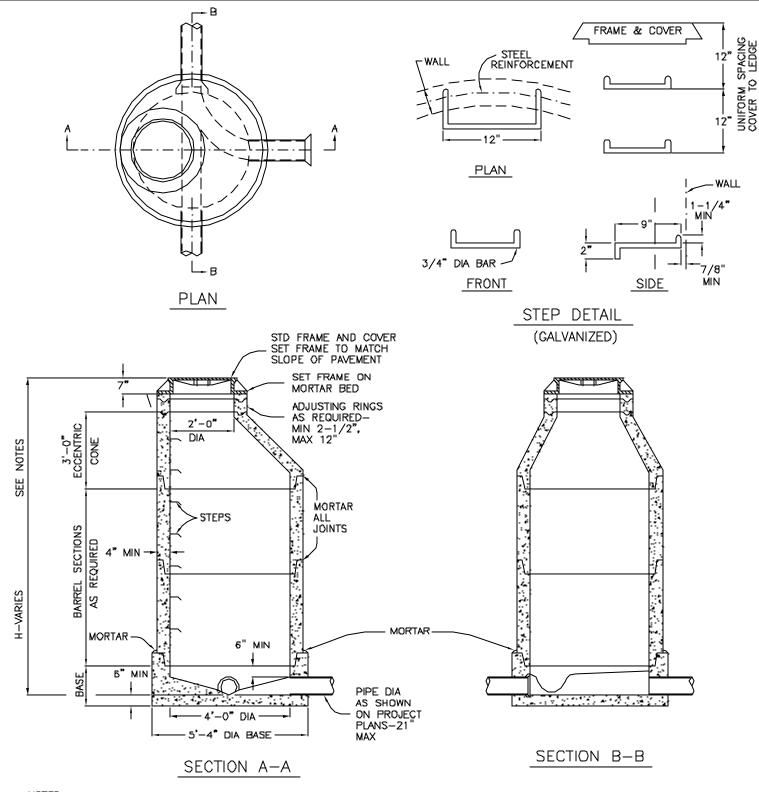
Marion County Dept. of Public Works Engineering Salem, Oregon



CATCH BASIN **GRATES AND FRAMES** TYPES A AND B

<u>20</u>

ı	Des.	Eng:	Date: 6-24-84	Revised:	
ı	Reviewed By	-Survey:	Datum: NONE	SCA	LES
<u>_</u>	Appd.	/nspec:	Drwn. CDM	Hor! NONE	Vert: NONE



- 1. PRECAST BARREL, CONE, AND EXTENSION RINGS SHALL BE REINFORCED CONCRETE MANHOLE SECTIONS CONFORMING TO ASTM C 478, AASHTO M199.
- 2. FOR STANDARD MANHOLE FRAME AND COVER DETAILS SEE DWG 23.
 3. FOR MANHOLE WITH "H" LESS THAN 4'-0" SEE PROJECT

- PLANS AND/OR DWG 21B.

 4. INSIDE JOINTS SHALL NOT EXCEED 3/8" IN THICKNESS.

 5. FORM CHANNELS IN MANHOLE BASE AS SHOWN.

 6. FOR LOCATION, PIPE SIZE, AND ELEVATION, SEE PROJECT
- PLANS. MAXIMUM PIPE SIZE 21".
 PLYWOOD FORM MANHOLE BASE.
 CONCRETE FOR BASE SHALL BE CLASS 3300 1—1/2" PER
 SECTION 504 OF THE STANDARD SPECIFICATIONS OF THE OREGON STATE HIGHWAY DIVISION.

Marion County Dept. of Public Works Engineering Salom, Oregon STORM DRAIN



/nspec:

		MΑ	ANHOLE			
	Eng:		Date: 5/25/94	Revised:		
wed By	-Survey:		Datum:		SCALES	

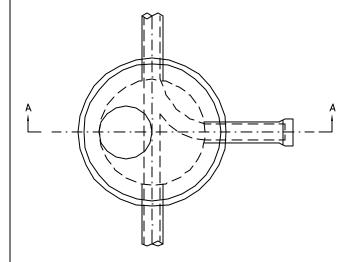
Hor: NONE

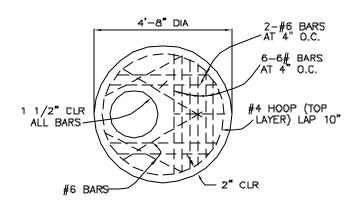
Drwn. CDM

21A

Vert: NOME

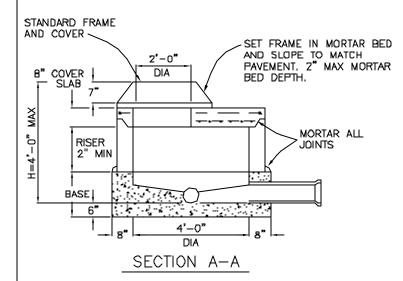
⁄:OF





PLAN (FRAME AND COVER NOT SHOWN)

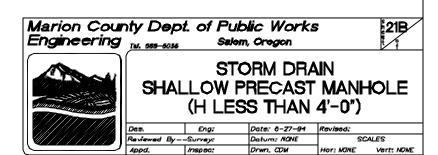
COVER SLAB REINFORCEMENT

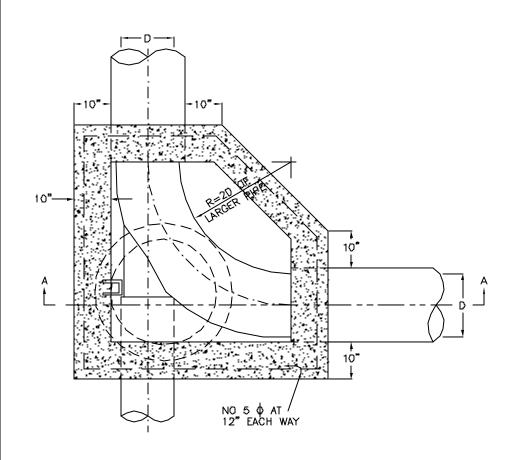


NOTES

- 1. PRECAST BARREL SHALL BE REINFORCED CONCRETE MANHOLE SECTION CONFORMING TO ASTM C478, AASHTO M199. 2. FOR STANDARD MANHOLE FRAME AND COVER DETAILS, SEE
- STD DWG 23.

 3. ALL CONCRETE SHALL BE CLASS 3300—1 1/2 PER SECTION 504 OF THE STANDARD SPECIFICATIONS OF THE OREGON STATE HIGHWAY DIVISION.
- 4. FORM CHANNELS IN MANHOLE AS SHOWN.
- 5. FOR LOCATION, PIPE SIZE AND ELEVATION, SEE PROJECT PLANS, MAXIMUM PIPE SIZE 21".
- 6. PLYWOOD FORM MANHOLE BASE.





SPECIAL BASE FOR MANHOLES AT ANGLE POINT IN LINE

PRECAST MANHOLE BARREL SECTIONS AS REQUIRED NO 5 Ф AT 12" /EACH WAY 2-1/2" 1 NO 5 0 HOOP -2-1/2" NO 5 \$\psi\$ AT \rightarrow 12" EACH WAY 6" OF 1"-0 CRUSHED ROCK BEDDING

SECTION A-A

<u>NOT</u>ES

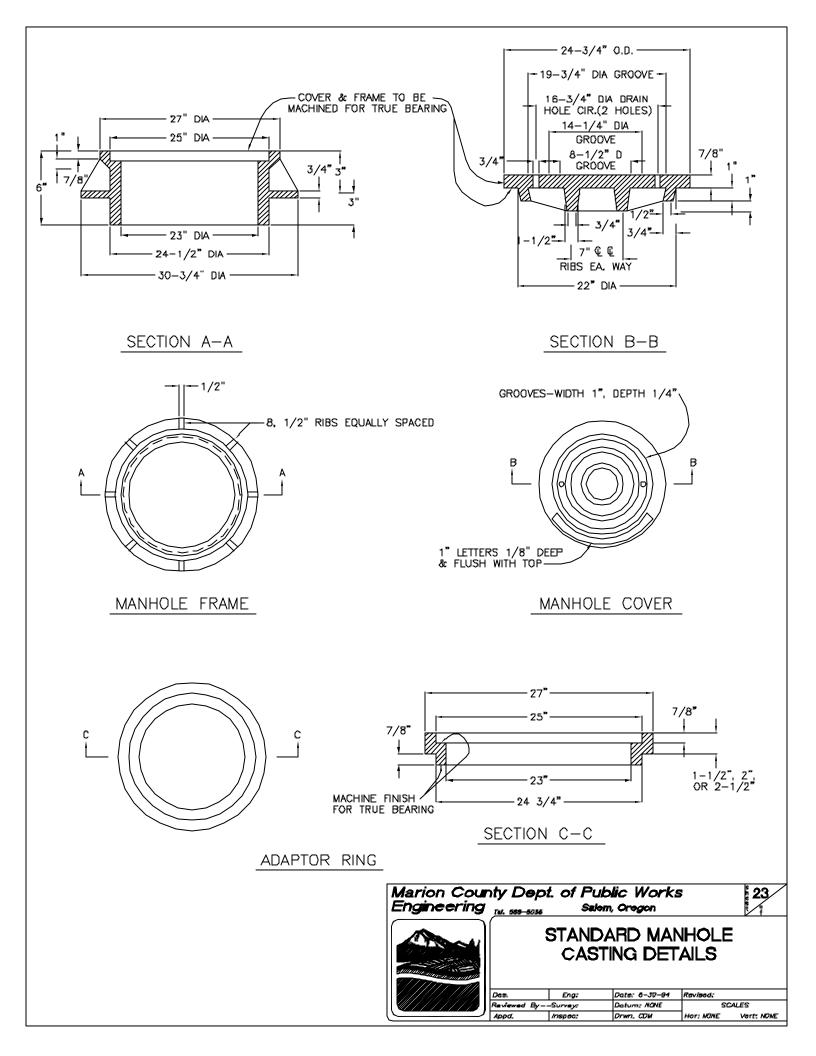
- 1. PRECAST MANHOLE BARREL SECTIONS, 1. PRECAST MANHOLE BARREL SECTIONS,
 ECCENTRIC CONE AND STANDARD
 FRAME AND COVER SHALL CONFORM
 TO STD DWG 21A.
 2. ALL CONCRETE SHALL BE CLASS 3300
 -1 1/2 PER SECTION 504 OF THE
 STANDARD SPECIFICATIONS OF THE
- OREGON STATE HIGHWAY DIVISION.
- 3. FORM CHANNELS IN MANHOLE AS SHOWN TO CONFORM TO INSIDE DIAMETERS OF PIPES.
- 4. PLYWOOD FORM MANHOLE BASE. 5. FOR LOCATION, PIPE SIZE AND ELEVATION, SEE PROJECT PLANS.

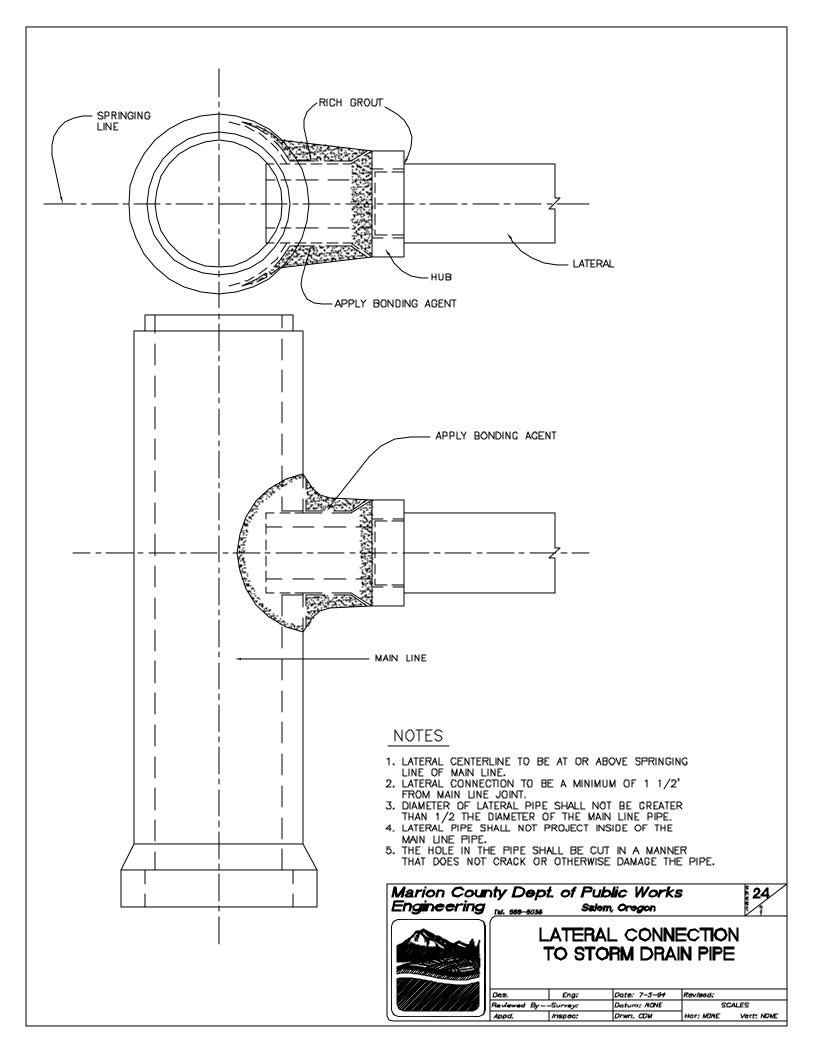
Marion County Dept. of Public Works Engineering Salom, Oregon

22

STORM DRAIN MANHOLE FOR PIPE 24' AND OVER

ı	Des.	Eng:	Date: 6-25-84	Revised:	
ı	Reviewed By	-Survey:	Datum: NONE	SCA	LES
<u>_</u>	Appd.	/nspec:	Drwn. CDM	Hor! NONE	Vert: NONE

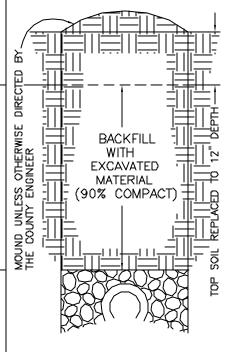


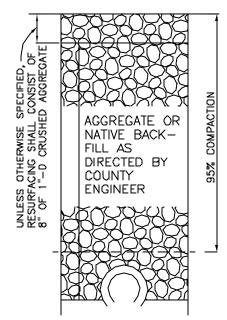


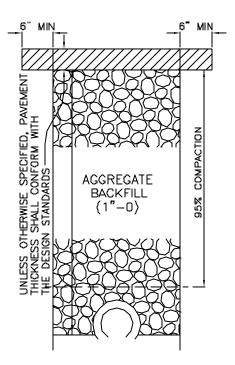
NATURAL OR OTHER THAN ROADWAY SURFACE (IN EASEMENT)

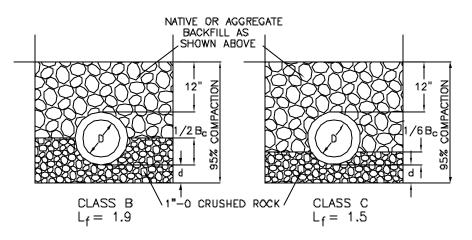
GRAVELLED OR DIRT ROAD SURFACE (COUNTY ROAD)

ASPHALT CONCRETE OR OILED GRAVEL SURFACE (COUNTY ROAD)









DEPTH OF BEI MATERIAL BELO	
D	D(WIN.)①
27" AND SMALLER	4"
30"- 60"	5"
66" and larger	6"

LEGEND

 $B_c = outside diameter$

= INSIDE DIAMETER

= DEPTH OF BEDDING MATERIAL BELOW PIPE

 $L_{\ell} = LOAD FACTOR$

NOTES

- 1. FOR ROCK OR OTHER INCOMPRESSIBLE MATERIALS, THE TRENCH SHALL BE OVEREXCAVATED A MINIMUM OF 6" AND REFILLED WITH AGGREGATE MATERIAL AS DIRECTED BY THE ENGINEER.
- 2. BEDDING AND BACKFILL MATERIALS IN THE PIPE ZONE SHALL BE COMPACTED AS SPECIFIED PRIOR TO BACKFILLING THE REMAINDER OF THE TRENCH.

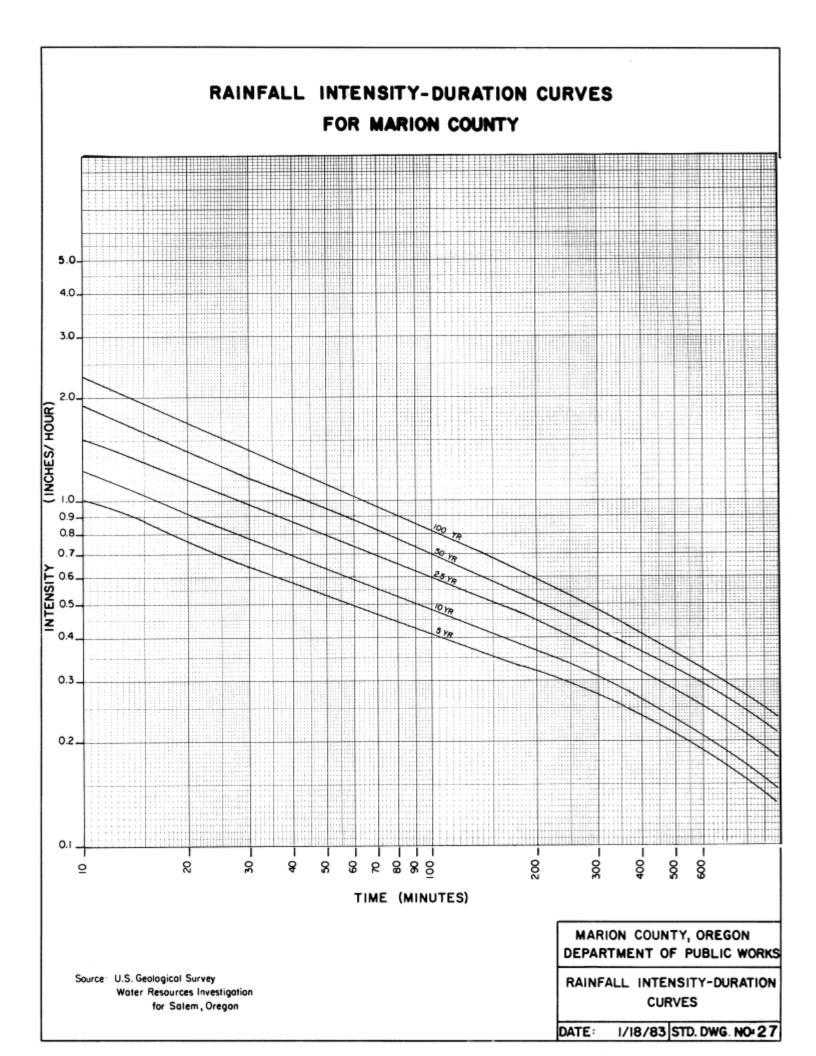




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					STORM SEWER	NER R	DESIGN SHEET	SHEET						
PROJECT				DESIGN	DESIGN FREQUENCY	ICY	-	DES	DESIGNED BY	-	DATE	-	PAGE	
STA	TLUNE -	DRAINAGE	BASIN	TOTAL	RUNOFF	AVERAGE	TIME	TOTAL	AVERAGE	DESIGN	INVERT	PIPE	CAPACITY VELOCITY	VELOCITY
	[±)	INDEX NO.	AREA (ACRES)	AREA (ACRES)		COEFF.	CONCEN. OF BASIN (MINUTES)	OF OF CONCEN. (MINUTES)	NTENSITY (NCHES/HR)	C.F.S.)		\sim	(C.F.S.)	(C.F.S.)
					STORM		SEWER DESIGN SHEET	HET.			DWG NO.). 26	-10L-30	06-JUL-94 cdm



SOIL CONSERVATON SERVICE TR-55 LAG-Tc METHOD PEAK DISCHARGE COMPUTATION SHEET

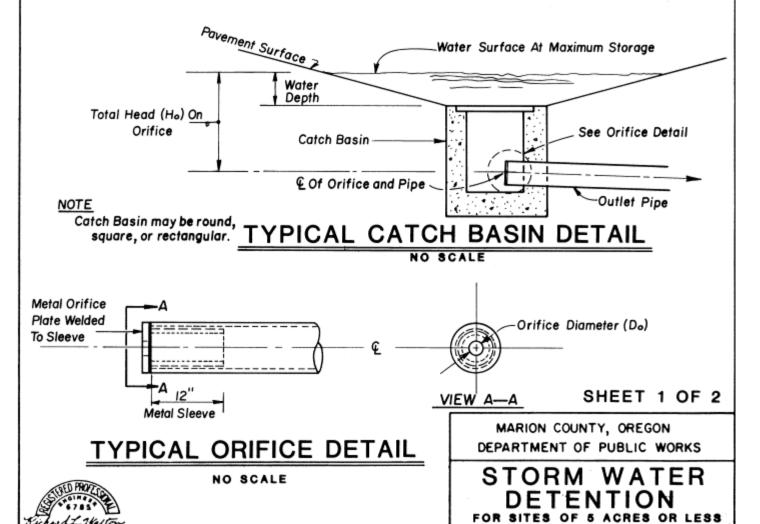
PF	ROJECT	WATERSHED C	ONDITION
	BY	DATE	
	CHECKED BY	DATE	
	INPUT		
1	(IN) (24 HOUR)	FIGURE 10	
1.		m	
	RAINFALL (MAP EXHIBIT 2—3A)	- <u>Tc</u>	
		<u>FACTOR</u>	
2.		1.67	
	RUNOFF CURVE NO. (EXHIBIT 2—2A)	CONSTANT	
	, ,	X	
3.	FT FIGURE 3-3 9.	- HR	
	HYDRAULIC LENGTH		
		BASIC LOG X	D 77 4 77
4.	% FIGURE 3-4		PEAK
	WATERSHED SLOPE 10.	╡	FACTOR
		HYDR. LENGTH ADJ.	13. IN
5 <i>.</i>	%	Χ	
Ο,	HYDR. LENGTH MODIFIED 11.		RUNOFF VOLUME
	THER, ELINOTH MODIFIED		X
-	~	IMP. AREA ADJ. F	1G. 2 14 CSM/IN
6.			BASIC PEAK DISCHARGE
	IMPERVIOUS AREA	——— HR	X
		$\overline{\text{Tc}}$	15.
7.	SQ MI		SQ. MI.
	DRAINAGE AREA (DA)		DRAINAGE AREA
			X
8.	/	, E-3, OR E-4	16.
	PONDS, SWAMPS (LOCATION DE	ETERMINES TABLE)	
			PONDS, SWAMPS ADJ.
	ADJUSTED PEAK I	DISCHARGE	17.
-			CFS
	PEAK DISCHARGE COMPUTATION SHE	ET	DWG NO. 28 7-6-94 cdm

ω	REQ. STORAGE (cu. FT.)																
RAGE																	
REQUIRED DETENTION STORAGE	INFLOW VOLUME OUTFLOW RATE OUTFLOW VOLUME (cu. FT.) (c.F.S.) (cu. FT.)																
ED DETEN	INFLOW VOLUME 0																
REQUII 4	RATE s.)																
DETERMINATION OF	RAIN INTENSITY (INCHES/HR)																
DETERMII 2	(ACRES)																
~	TIME (minutes)																
DET	ΓERM	INATION	OF I	REQU	JIRED) DE	TENT	ION	STO	RAGE	<u>-</u> -	D	WG	NO.	29	Б-7—I	94 cdm

Area of Developed Site (acres)(1)	Allowable Outflow (cubic feet per second)	Orifice Diameter (inches)(2)	Volume of Water to Be Stored (cubic feet)	Water Depth Over Inlet Grate (feet)(3)	Water Storage Area (square feet)(3)
0.5	0.10	1-11/16	780	0.5	4,690
1.0	0.20	2-3/8	1,560	•	9,380
1.5	0.30	2-7/8	2,350	•	14,070
2.0	0.40	3-5/16	3,130		18,760
2.5	0.50	3-11/16	3,910		23,450
3.0	0.60	4	4,690		28,150
3.5	0.70	4-3/8	5,470		32,830
4.0	0.80	4-11/16	6,250		37,520
4.5	0.90	4-15/16	7,040		42,220
5.0	1.00	5-3/16	7,820	•	46,910

- Por areas less than 0.5 acre, detention is not required. For areas greater than 5.0 acres, the detention system must be designed on a site-specific basis with an allowable outflow based on a 5-year storm with a runoff factor of 0.20 and storage for a 10-year storm with a runoff factor of 0.90.
- Orifice diameter (D_o) is based on the allowable flow (Q_o) and an assumed total head (H_o) on the orifice of 2.00 feet (see typical details). If the total head is different, the diameter must be determined from the graph on Sheet 2.
- 3 If site conditions necessitate the use of a different water storage area, the water depth must be calculated and an orifice diameter determined per Note 2 above. In most cases, the following formula can be used for calculating the depth:

Water Depth = 3 x Volume of Stored Water : Water Storage Area



REV. DESCRIPTION

DATE

DRAWN BY:

APPVD.

APPROVED BY:

MWS

DATE: 10/10/85

DWG. NO.: 30

ORIFICE DIAMETER (D.) inches TOTAL HEAD ON ORIFICE (H_o) feet 4.0 3.0 2.0 1.0 0.1 3.0 ALLOWABLE OUTFLOW (Q.) cubic feet per second

SHEET 2 OF 2

MARION COUNTY, OREGON DEPARTMENT OF PUBLIC WORKS

STORM WATER

DATE: 10/10/85

30 DWG. NO.: APPROVED BY: REV. DESCRIPTION

